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An holistic approach to beach erosion vulnerability assessment

SUBJECT AREAS:

CLIMATE-CHANGE
IMPACTS

GEOMORPHOLOGY

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Erosion is a major threat for coasts worldwide, beaches in particular, which constitute one of the most valuable coastal landforms. Vulnerability assessments related to beach erosion may contribute to planning measures to counteract erosion by identifying, quantifying and ranking vulnerability. Herein, we present a new index, the Beach Vulnerability Index (BVI), which combines simplicity in calculations, easily obtainable data and low processing capacity. This approach provides results not only for different beaches, but also for different sectors of the same beach and enables the identification of the relative significance of the processes involved. It functions through the numerical approximation of indicators that correspond to the mechanisms related to the processes that control beach evolution, such as sediment availability, wave climate, beach morphodynamics and sea level change. The BVI is also intended to be used as a managerial tool for beach sustainability, including resilience to climate change impact on beach erosion.

Beaches are by nature unstable coastal landforms as they respond to changes in sediment supply, nearshore hydrodynamics and sea level. Across Europe, coastal erosion has been a longstanding, large-scale issue¹ with more than 40% of the beaches in France, Italy and Spain being under erosion². Similarly, in the USA³ of the 33,000 km of eroding shoreline, some 4,300 km are beaches³. Moreover, beach erosion poses a major threat not only to interconnected ecosystems⁴, but also to stakeholders, as it is related to beach property values⁵ and tourism⁶.

Beach evolution depends on processes such as sediment availability⁷, storms causing changes that persist with time⁸, complex interactions between nearshore and onshore sedimentary bodies⁹, sea level rise^{10,11} and the broader coastal geological setting¹². Erosion, on the other hand, is usually the combined result of a wide range of factors, both natural (e.g. winds, storms, nearshore currents) and human-induced (e.g. coastal engineering, river basin regulation) that operate on different time and spatial scales¹³.

Thus, quantification of these factors becomes more difficult due to their variability and coupling of the processes that affect coastal areas, and also to the frequency at which coastal changes occur¹⁴. The estimation of the vulnerability of coastal areas to erosion has received considerable attention and a vast literature is available in this field; however, most of this is associated with sea level rise, induced by climate change^{15–18}. In the case of beaches, early attempts were based upon simple approaches^{19–21}, focusing on erosion due to sea level rise. More recently, methods estimating vulnerability associated with storms have been developed^{22–26}.

To this direction, we present an index dedicated to the assessment of vulnerability to erosion exclusively of beaches, considering the predominant hydro- and sediment- dynamic processes that contribute to beach evolution. Moreover, the new Beach Vulnerability Index (BVI) refers to beaches, regardless their size, and to the features of the associated coastal environments, incorporating processes operating over long (e.g. a gradual change in sea level) and short (i.e. storm events) time periods. Special effort has been placed upon the index estimation in order to require easily obtainable data and to avoid calculations demanding high processing capacity.

Index Development

The idea of the Beach Vulnerability Index (BVI) originates from the Coastal Vulnerability Index developed by Gornitz et. al.¹⁵. It is based on a numerical approximation of the principal physical processes that control the evolution of a beach; these, in turn, are related to sediment availability (terrestrial, aeolian and marine), nearshore hydro- and sediment- dynamics and relative sea level change²⁷. Subsequently, we have identified the following mechanisms to consider as indicators that are related to the aforementioned processes: (i) long-shore sediment transport; (ii) cross-shore sediment transport; (iii) riverine sediment inputs; (iv) the effect of sea level change; (v)



erosion of associated coastal landforms; (vi) wave run up; and, (vii) aeolian transport. The calculation of these mechanisms incorporates a large number of parameters related to nearshore hydrodynamics (e.g. wave breaking characteristics, energy flux within the nearshore zone), sediment dynamics (e.g. threshold of sediment movement, beach morphodynamic characteristics), aeolian transport of beach material (e.g. wind speed and direction, beach grain size) and the terrestrial supply of sediment (e.g. riverine sediment influx, advection of erosion products from neighbouring coastal landforms). For the numerical estimation of the above mechanisms existing data-sets (e.g. wind/wave climate), simple *in-situ* measurements (e.g. beach profiling), grain-size analysis and common numerical modelling (e.g. wave refraction, wave breaking characteristics) could be utilised. Thus, the BVI expands its applicability as its estimation does not need massive or difficult to be collected, data or demanding computing capacity.

The B.V. Index calculation follows a four step procedure: (1) identification of beach physiographic, climatic and oceanographic characteristics; (2) division of the beach in characteristic sectors, according to step 1; (3) calculation of vulnerability indicators; and, (4) index estimation (figure 1).

The first step includes the identification of geomorphological and oceanographical characteristics of the beach, together with climatic conditions of its hinterland. The geomorphological characteristics refer to nearshore bathymetry, beach slope, width and granulometry. Beach profile characteristics (slope, profile length) are estimated by taking into account tidal range²⁸, while the oceanographic characteristics refer to incoming waves (significant height, period and length, closure depth) and those at wave breaking (height, angle and depth). Climatic conditions are considered also for aeolian transport (wind speed and direction). In the case of riverine influxes, the climatic (air-temperature, precipitation) and hydrological characteristics of the catchment area are included. For the parameterisation of the aforementioned characteristics that control sediment availability, four categories of data are required (figure 1), i.e. morphological, sedimentological, climatic and hydrodynamic.

In the second step, the beach is divided into alongshore sectors, with width that is dependent upon the required resolution in each case study and on the basis that each sector is homogenous in terms of geomorphological (e.g. the presence of a river mouth or dune field), sedimentological (different granulometry) and wave climate characteristics (e.g. due to varying shoreline orientation). In the calculations, each sector is represented by a profile aligned normally to the shoreline, with mean tidal elevation defining shoreline position. The landward upper limit of the beach profile may be either a morphological change (e.g. the base of a coastal cliff, or a dune), or a human construction (e.g. coastal wall). In order to define the seaward limit of each sector, the approach of defining the closure depth is adopted; the latter is calculated using the Hallermeier²⁹ formula, which basically depends on the storm wave characteristics that are considered to be associated with the maximum observed waves.

$$h_c = 2.28H_e - 68.5 \left(\frac{H_e^2}{gT_e^2} \right) \quad (1)$$

where, g is the acceleration of gravity (m/s^2), H_e is the storm wave height before breaking (m), and T_e is the corresponding wave period.

The third step addresses the calculation of the seven (7) indicators (mechanisms) that correspond to the aforementioned processes and are used for the estimation of the index. Each indicator is defined probabilistically by using the weighted average value with respect to the frequency of occurrence for a 30-year period. This is similar to the way that Bosom and Jiménez²³ estimate the probabilistic vulnerability of beaches to storm events. Each indicator is characterised as positive if it indicates sediment loss and negative if it involves sediment gain. The homogenisation of all indicators is achieved by converting them into non-dimensional values, dividing their average

value with their maximum values and expressing the ratios produced as percentages.

Indicators

The calculation of the seven indicators is based on the application of mathematical relationships, which have been selected after several tests considering their acceptability, reliability, simplicity, and data demand.

The **Long-shore sediment transport (Q_l)** indicator is estimated by the ratio:

$$Q_l = 100 \frac{Q_{l \text{ WHA}}}{Q_{l \text{ max}}} \quad (2)$$

where, $Q_{l \text{ max}}$ is considered to be the potential volumetric longshore transport rate and $Q_{l \text{ WHA}}$ corresponds to the weighted average value with respect to the frequency of wave occurrence; the latter is justified by the wave heights that currents with velocities exceeding the threshold for sediment movement³⁰ can produce:

$$U_{wle} = 2\pi C \left[1 + 5 \left(\frac{T_r}{T_o} \right)^2 \right]^{-1/4} \quad (3)$$

where, T_o is the period of the incoming offshore waves, while C and T_r are coefficients deriving from s_* , a scaled dimensionless immersed sediment weight, provided by the following relationships:

$$C = \frac{2.53 v s_*^{0.92}}{4 D_{50}} \quad (4)$$

$$T_r = \frac{159 s_*^{-1.3} D_{50}}{v} \quad (5)$$

$$s_* = \frac{D_{50} \sqrt{\rho_s g D_{50}}}{4v} \quad (6)$$

where, U_{wle} is critical boundary velocity (m/s); T_o is wave period (m), D_{50} is grain size (mm); v is kinematic viscosity of seawater ($\sim 1 \cdot 10^{-6} m^2/sec$); and ρ_s is sediment density (gr/cm^3).

To estimate the $Q_{l \text{ WHA}}$ and $Q_{l \text{ max}}$ values, the Komar³¹ formula has been used, since it provides a very good estimation for longshore sediment transport, with the only requirement being that of wave breaking characteristics:

$$Q_l = 1.1 \rho g^{3/2} H_b^{5/2} \sin a_b \cos a_b \quad (7)$$

where, Q_l is potential volumetric longshore transport rate (m^3/day), ρ is water density (gr/cm^3), g is acceleration of gravity (m/sec^2), H_b is breaking wave height (m), and a_b is wave breaking angle.

The **Cross-shore sediment transport (Q_c)** indicator is given by the ratios:

$$Q_c = \begin{cases} 100 \frac{Q_{c \text{ WHA}}}{Q_{c \text{ max}}} (\text{offshore}) \\ 100 \left(1 - \frac{Q_{c \text{ WHA}}}{Q_{c \text{ max}}} \right) (\text{onshore}) \end{cases} \quad (8)$$

where, $Q_{c \text{ max}}$ is potential volumetric cross-shore transport rate, and $Q_{c \text{ WHA}}$ is the cross-shore sediment transport for those wave heights that exceed the corresponding threshold of the critical boundary velocity (U_{wle}), in respect to the frequency of occurrence. The direction of sediment movement is estimated by the use of the criterion³², given by the following relationship, while offshore values are regarded as positive, and onshore values as negative.

$$\frac{g H_o \tan \beta T}{L_o w_s} \begin{cases} > 0.5 (\text{offshore}) \\ < 0.5 (\text{onshore}) \end{cases} \quad (9)$$

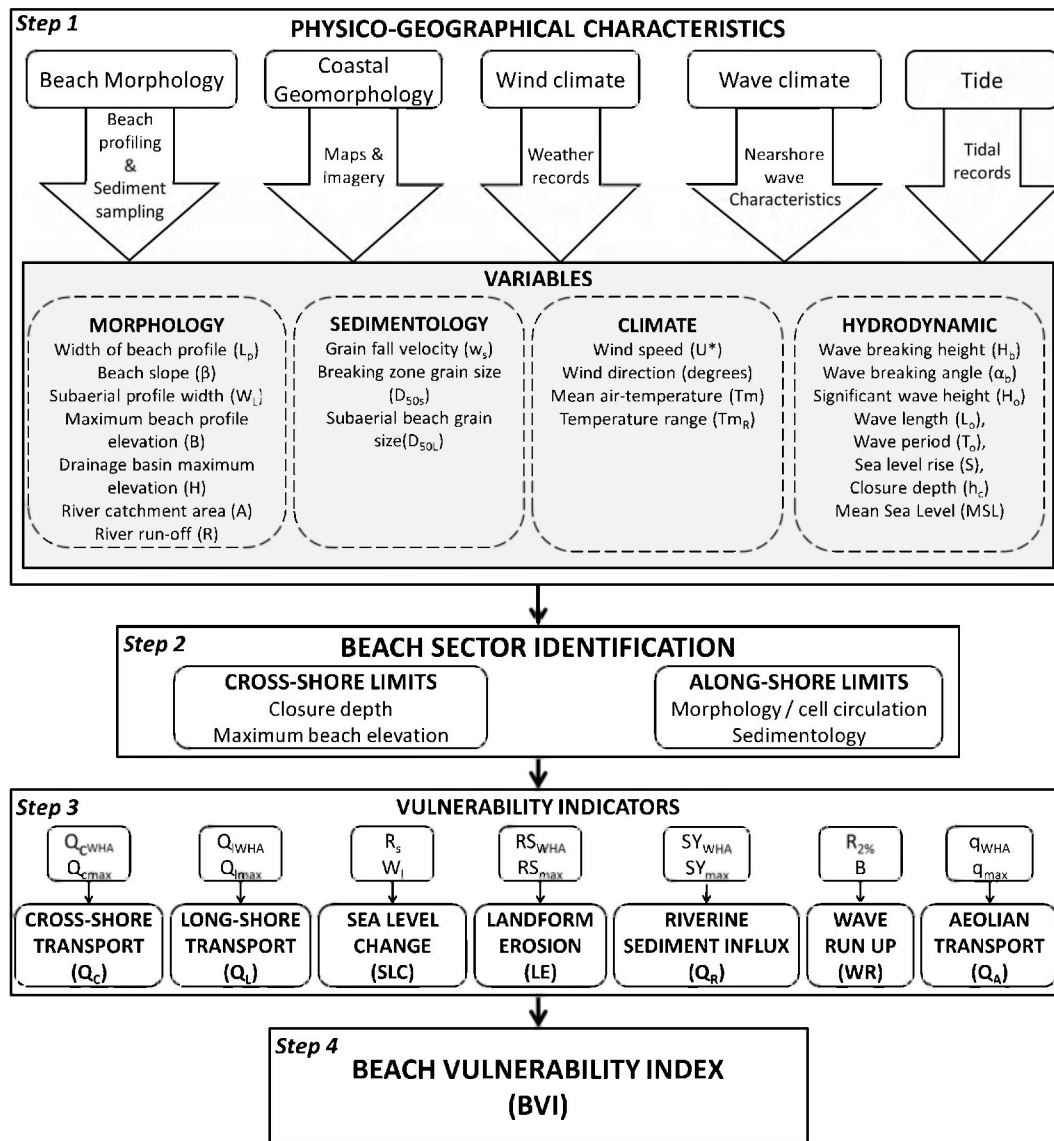


Figure 1 | Flow chart of the calculation method for the Beach Vulnerability Index.

The values for the indicator expressing the cross-shore sediment transport (Q_c) are calculated with the use of the equation³³:

$$Q_c = \rho C_D u_m^3 \left\{ \frac{B}{\tan \varphi} \left(\psi_1 + \frac{2}{3} \delta_u - \frac{\tan \beta}{\tan \varphi} u_s^* \right) + \frac{u_m}{w_s} \left[\psi_2 + \delta_u u_3^* - \frac{u_m}{w_s} \varepsilon_s u_s^* \tan \beta \right] \right\} \quad (10)$$

where, $\varepsilon_B = 0.2$; $\varepsilon_S = 0.025$; C_D = drag coefficient; w_s = sediment fall velocity (m/s); φ = the angle of repose; β = the beach slope; u_b = near bed water velocity (m/s); ρ = water density (gr/cm³); and, ρ_s = density of sediment (g/cm³); δ_u , ψ_1 , ψ_2 , u_m , u_3^* , and u_s^* = cross-shore velocities (m/s).

The cross-shore (δ_u , ψ_1 , ψ_2 , u_3^* and u_s^*) variables expressed as a function of significant wave height are given by the relationships³⁴:

$$\psi_1 = 0.303 - 0.00144 H_o \quad (11)$$

$$\psi_2 = 0.603 - 0.00510 H_o \quad (12)$$

$$\delta_u = 0.458 - 0.00157 H_o \quad (13)$$

$$u_m = 31.9 + 0.403 H_o \quad (14)$$

$$u_3^* = 0.548 + 0.000733 H_o \quad (15)$$

$$u_s^* = 1.5 + 0.00346 H_o \quad (16)$$

The Bailard and Inman formula³³ was used because it is the only one that does not require detailed field data and it can provide satisfactory results³³.

The Riverine sediment influx (Q_R) indicator is given by the ratio:

$$Q_R = 100 \left(1 - \frac{SY_{WHA}}{SY_{max}} \right) \quad (17)$$

where, the values SY_{WHA} and SY_{max} correspond to weighted average values, for a 30-year period, in respect to the frequency of occurrence and maximum freshwater discharge values respectively. Q_R receives a negative sign in calculations, as it corresponds to sediment gain. For the estimation of river sediment flux, the Hovius formula³⁵ is used:

$$\ln SY = -0.416 \ln A + 4.26 \cdot 10^{-4} H + 0.15 H + 0.15 T_m + 0.095 T_{mR} + 0.0015 R + 3.58 \quad (18)$$



where, A is the catchment area (m^2), H is the maximum elevation of the drainage basin (m), T_m is the mean temperature ($^{\circ}C$), T_{mR} is temperature range ($^{\circ}C$), and, R is river run-off (m^3/s) (all attributes are treated as dimensionless).

In cases where there is no sediment supply from riverine systems, the indicator reaches its maximum value (i.e. 100). This method of estimating riverine sediment influx, although does not take into consideration sediment availability in the catchment area, was depicted after it was tested with similar formulae^{36,37} and was found to produce good results³⁸. In addition, it satisfies the criterion of broad applicability, as all the incorporated attributes related to the catchment area are easily acquired.

The **Coastal Landform Erosion (LE)** indicator expresses the contribution of coastal landform erosion in the displacement of the shoreline. It is given by the ratio:

$$LE = 100 \left(1 - \frac{RS_{WHA}}{RS_{max}} \right) \quad (19)$$

where, RS is the difference between the present (R_1) and future (R_2) shoreline position, RS_{WHA} and RS_{max} corresponds to the weighted average value in respect to the frequency occurrence of the prevailing wave and storm wave conditions, respectively. For the calculation of the RS , the following formula³⁹ is used:

$$RS = R_2 - R_1 = \frac{(S_2 - S_1)L_p}{p(B + h_c)} \quad (20)$$

where, S_1 < S_2 , expressed in metres, correspond to historical and expected sea level rise, respectively, L_p is the profile length (m), B is berm height (m), P is the proportion of coarse sediment that is adequate to retain the shore profile in equilibrium, and h_c is closure depth (m).

The selection of this formula is based on the comparison of its estimates to the results of other more detailed calculations, such as that provided by the SCAPE model⁴⁰.

The **Sea Level Change (SLC)** indicator is given by the relationship:

$$SLC = 100 \left(\frac{R_s}{W_1} \right) \quad (21)$$

where, R_s corresponds to the weighted average conditions in respect to frequency occurrence and W_1 is the beach width.

The shoreline retreat due to relative sea level change is given by the Dean semi-empirical relationship²⁰, which produces better results than others⁴¹, by incorporating storm surge and wave set-up variables:

$$R_s = (S + 0.068H_b) \frac{W}{d_b + B} \quad (22)$$

where, B is berm height (m), H_b is wave breaking height (m), S is relative sea level rise (m), W is profile length (m), and d_b is breaking depth (m).

The selected equation 22 presents certain deficiencies, as it originates from the Brunn rule⁴² that has been extensively criticised, due to the assumptions involved and because it omits many important variables⁴³. On the other hand, it seems to provide satisfactory results, when compared to other static or dynamic approaches⁴⁴. Therefore, it is considered adequate to quantify this indicator, keeping in mind that variables which control erosion rates are addressed in the index by other indicators.

The **Wave Run-up (WR)** indicator is provided by the ratio:

$$WR = 100 \left(\frac{R_{2\%}}{B} \right) \quad (23)$$

where, $R_{2\%}$ is wave run-up for the 2% of maximum incoming waves, whilst the maximum beach elevation (B) is considered as maximum value.

The estimation of $R_{2\%}$ has been made with the use of the Stockton's et. al., formula⁴⁵, since it has been proved in other cases, that it provides better results than other approaches on the basis of field measurements and coastal imaging analysis⁴⁶.

$$R_{2\%} = 1.1 \left(0.35 \beta \sqrt{\frac{H_o}{L_o}} + \frac{[H_o L_o (0.563 \beta^2 + 0.004)]^{1/2}}{2} \right) \quad (\text{all beaches}) \quad (24)$$

$$R_{2\%} = 0.043 \sqrt{H_o L_o} \quad (\text{dissipative beaches}) \quad (25)$$

where, H_o is offshore significant wave height (m); L_o is offshore wave length (m), and β is beach slope (in rads).

The indicator related to **aeolian transport (Q_A)** with respect to the direction of movement is given by the ratios:

$$Q_A = \begin{cases} 100 \frac{q_{WHA}}{q_{max}} & (\text{seawards}) \\ 100 \left(1 - \frac{q_{WHA}}{q_{max}} \right) & (\text{landwards}) \end{cases} \quad (26)$$

The aeolian transport rate q (in $gm/cm^2/s$) is estimated using the Hsu empirical formula⁴⁷, since it has proved to have very good correlation with field measurements⁴⁸.

$$q = V_a P_a \left(e^{-0.63 + 0.91 D_{50L}} \right) \left[\sqrt{\frac{U^*}{g D_{50}}} \right] \quad (27)$$

where, U^* is shear velocity (m/s), g is acceleration of gravity (m^2/s), D_{50} is mean grain size (mm), V_a is air kinematic viscosity (m^2/s), and ρ_a is mass air density (g/cm^3).

The mean (q_{WHA}) transport rates correspond to the weighted average conditions with respect to wind frequency of occurrence, while the maximum (q_{max}) rates correspond to maximum values of shear velocity (U^*). The threshold for the aeolian sediment transport is provided by the formula⁴⁹:

$$U_{ut} = A_t \sqrt{\frac{(P_s - P_a) g D_{50}}{P_a}} \quad (28)$$

where, U_{ut} is critical boundary air velocity (m/s), A_t is a dimensionless coefficient equal to 0.118, and ρ_s is mass density of the sediment (g/cm^3).

Index calculation

We have calculated Beach Vulnerability Index (BVI) as being the arithmetic mean of the above mentioned vulnerability indicators:

$$BVI = \frac{Q_1 + Q_c + LE + Q_A + WR + SLC + Q_R}{7} \quad (29)$$

The arithmetic mean was chosen after several tests were conducted with other types of statistical mean (e.g. geometric mean, root mean square) and after considering the different approaches adopted in the calculation of other indices. Therefore, this approach, considering all indicators of equal importance, reduces, not only subjectivity⁵⁰, but it also permits identification of the relative significance of index indicators.

Subsequently, the index values are expressed as percentages (0-100%) and ranked into five classes, following the application of normal distribution to the total number of beach sectors involved in the analysis. The five statistical classes produced, on the basis of the mean value (μ) and standard deviation (σ), correspond to the five categories of beach vulnerability (figure 2): very low (1); low (2); medium (3); high (4) and very high (5).

Data requirements. The data needed for the calculation of index indicators could be distinguished in two main categories: (i) raw data, deriving from maps (e.g. topographic), aerial photographs

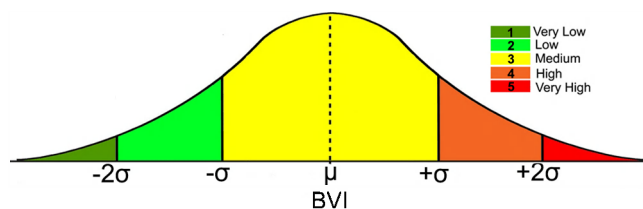


Figure 2 | The five (5) categories of Beach Vulnerability Index (μ is the geometric mean; and σ is the standard deviation).

and satellite images, obtained by field observations and measurements (e.g., shore normal profiles) and laboratory analyses (e.g., grain size); and (ii) analytical data, produced by analysing the above mentioned raw data as well as those gathered from literature search (e.g. wind data). In figure 1, the required data for the BVI application is presented analytically. As aforementioned, the calculation of index indicators has aimed to satisfy both data availability and low economic resource for obtaining the required data, while their analyses aspire to be accomplished using reasonable computing efforts (see also Methods section). Moreover, it is worth mentioning that some countries have started to develop databases with the required data sets (e.g. the U.K.⁵¹).

Index testing

We have applied the Beach Vulnerability Index to 18 beaches, divided in 138 beach sectors, around the Greek coastline (Figure 3a) that are characterised by different physio-geographical characteristics, in terms of associated coastal landforms (sand dunes, deltas, lagoons, coastal cliffs), incoming wave regime (open coast, semi-protected), geological setting and beach material (i.e., grain size). The mean values of the seven indicators, together with the overall classification of their vulnerability, are presented in table 1, while a graphical output for the beach of Agia Anna (location 14) is presented in figure 3b. In the case of Agia Anna beach (figure 3b) the index indicators are associated with significantly different values over the nine (9) beach sectors. Thus, the mean longshore transport indicator (Q_l) is estimated to be 15.08, but it is associated with zero value in Sectors 2, 6 and 7; Sector 3 presents the maximum estimated

value (29.30). For the cross-shore (Q_c) indicator, the mean value is 41.62, varying from 31.64 (Sector 1) to 46.77 (Sector 3). Wave run-up (WR) values range from 13.45 (Sector 7) to 64.88 (Sector 9), with the mean value being 26.92. The Aeolian transport (Q_A) indicator has a mean value of 8.73, being zero in all sectors except in Sectors 3 (38.21) and 4 (40.35). The sea level change indicator (SLC) gives values, which vary from 2.24 (Sector 3) to 9.61 (Sector 1), whilst its mean value is 3.43. The riverine inputs indicator (Q_R) has a maximum value (100) in Sectors 1–4, as these sectors do not receive the river Boudouros influxes. In Sectors 5 and 6, Q_R presents values 57.59 and 56.70, respectively, while in the northern Sectors 7 to 9 it is characterised with very low values (<4). The land erosion indicator (LE) has a minimum value in Sector 3 (35.12) and a maximum value in Sector 9 (72.49), while the mean value of 58.69. Finally, BVI values vary from 19.51 (Sector 1) to 37.97 (Sector 4), having a mean value of 30.09. On the basis of the statistical analysis for the 138 sectors of the 18 beaches included in the analysis, the values justifying the 5 categories of vulnerability, on the basis of their median (33.96) and standard deviation (11.38) are: very low (<11); low (11–22); medium (34–45); high 34–45; very high (>45). Hence, Sectors 1, 2 and 5–9 are subjected to moderate vulnerability (29.0–38.8), whilst Sectors 3 and 4 to are subjected to low vulnerability (19.2–29.0).

In most of the study areas the riverine influx indicator attains its maximum value (100), indicating high vulnerability. This is due to the fact that most of the beaches involved in this analysis do not host active river mouths, while in a few cases either sediment influx is retained behind dams or river mouths have been subjected to artificial regulation (e.g. offshore channel prolongation).

Index results and beach behavior patterns. The results of the BVI were found to be in very good correlation with the behaviour patterns of the beaches involved in the analysis. Characteristically, we refer to three beaches with different physico-geographical characteristics (figure 4). In the case of Almiros beach (location 2, figure 3), vulnerability values are increasing from west to east, suggesting higher erosion rates eastwards; this is in agreement with the observations that the the eastern and central part of the Almiros bay have retreated up to 15 m during the past 25 years, while its western part seems to be rather stable⁵². In the case of Ammoudara

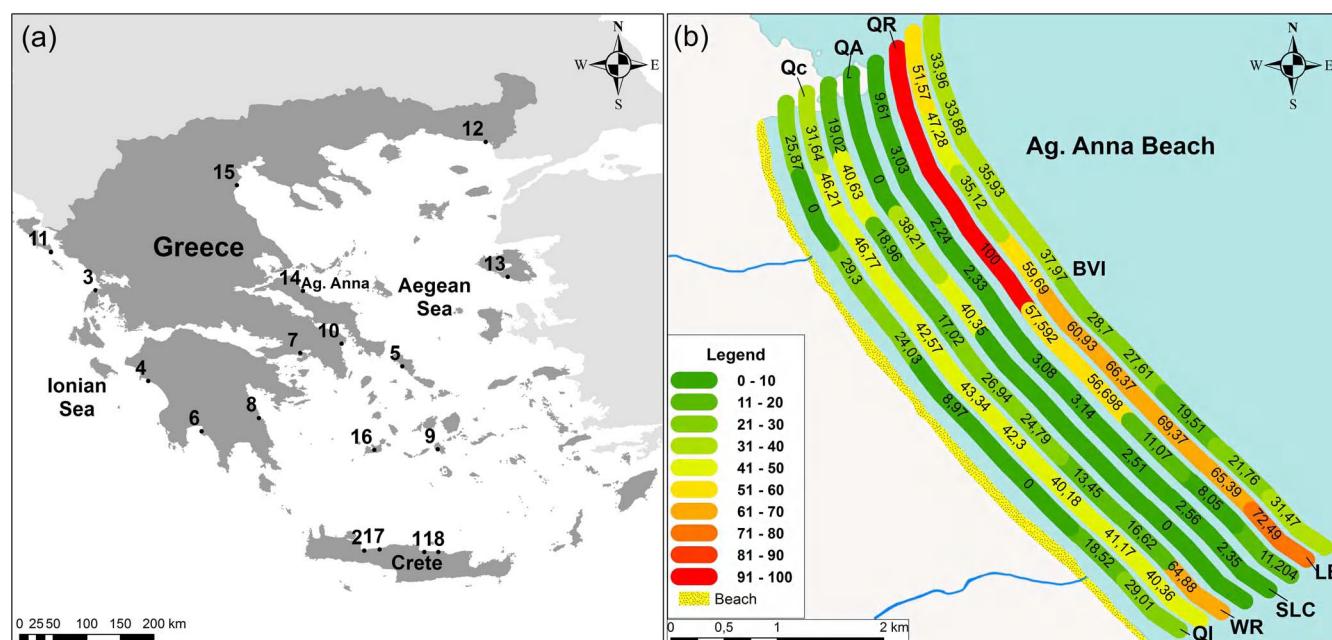


Figure 3 | The locations of the case study areas mentioned in table 1 (a); and graphical presentation of BVI application in the case of the Ag. Anna beach (b). Maps were created with ArcMap 9.3.



Table 1 | Mean values of the indicators used in the BVI calculation, together with their vulnerability ranking

Beach	No Sectors	Q _L	Q _C	WR	Q _A	SLC	Q _R	IE	BVI	Rank
1 Ammoudara	13	4.44	11.30	13.81	21.84	11.81	48.82	73.37	26.48	Medium
2 Almiros	8	6.61	10.51	31.22	31.21	10.92	12.52	36.72	19.96	Low
3 Ag. Ioannis	11	11.53	42.65	6.48	18.00	5.55	100	16.32	28.55	Medium
4 Alfios Delta	4	13.72	29.85	43.00	4.75	27.46	42.19	38.29	28.38	Medium
5 Ag. Petros	3	13.82	26.13	35.87	36.15	70.62	100	38.04	44.95	High
6 Santava	5	8.71	40.00	29.59	18.94	26.93	100	84.24	42.71	High
7 Kinetta	11	11.73	45.46	48.53	26.75	46.77	100	37.84	43.39	High
8 Astros	6	18.67	31.79	63.11	1.81	55.75	100	55.14	46.48	Very high
9 Milopotamos	5	22.68	17.81	54.78	25.73	5.40	100	83.16	42.38	High
10 Marathon	8	22.25	14.75	48.99	43.77	22.63	100	59.00	41.36	High
11 Korission Lagoon	10	13.66	22.81	18.74	22.33	22.63	100	47.07	33.73	Medium
12 Alexandroupolis	3	11.30	17.28	48.47	25.84	9.00	100	27.43	33.57	Medium
13 Vatera	11	26.75	18.95	56.82	5.07	8.36	25.07	82.98	31.64	Medium
14 Ag. Anna	9	15.08	41.62	26.92	8.73	3.43	60.51	58.69	30.09	Medium
15 Skala Katerinis	4	12.47	26.98	79.42	29.70	23.21	33.18	73.16	37.61	High
16 Achivadolimni	3	23.09	18.91	20.85	44.04	6.89	100	80.94	38.96	High
17 Rethymnon	15	15.82	60.32	21.28	46.46	29.31	72.10	54.58	42.59	High
18 Gouves	9	8.10	38.07	64.01	35.04	57.38	100	100	57.18	Very high

beach (location 1, figure 3), BVI attains values between 20 and 27, suggesting a rather homogenous behavior involving small rates of erosion, which is in agreement with the findings of a recently published work⁵³, wherein it is stated that from 2005 to 2012 the shoreline has retreated uniformly only a few meters. Finally, BVI for the Vatera beach (location 13, figure 3) shows high vulnerability values in its central sectors (45.38), compared to both outer parts (30.81 and 27.44), with the former sectors being associated with shoreline retreat in the order of 4–5 m⁵⁴, when the latter seems to be rather stable.

Concluding comments

The advantage of the developed BVI, compared to other similar vulnerability assessment methods applicable to beaches⁵⁵, which usu-

ally deal with one process (i.e. storm impact²³) lies upon its holistic approach to the processes controlling beach evolution; the latter can be represented numerically and integrated quantitatively and qualitatively. Moreover, the equal contribution of the seven (7) indicators not only diminishes the subjectivity in weighting index's indicators⁵⁰, but also reveals the principal processes through the quantitatively identification of their relevant significance in beach evolution.

The Beach Vulnerability Index could be improved further, in terms of numerical estimations of its indicators, in terms of both data and processing capacity, as it operates independently to index ranking. On the other hand, the index could incorporate more advanced methods (such as numerical models) for more accurate calculation of index variables. But, this will require more data and processing effort, making the Index less functional for non-expert users.

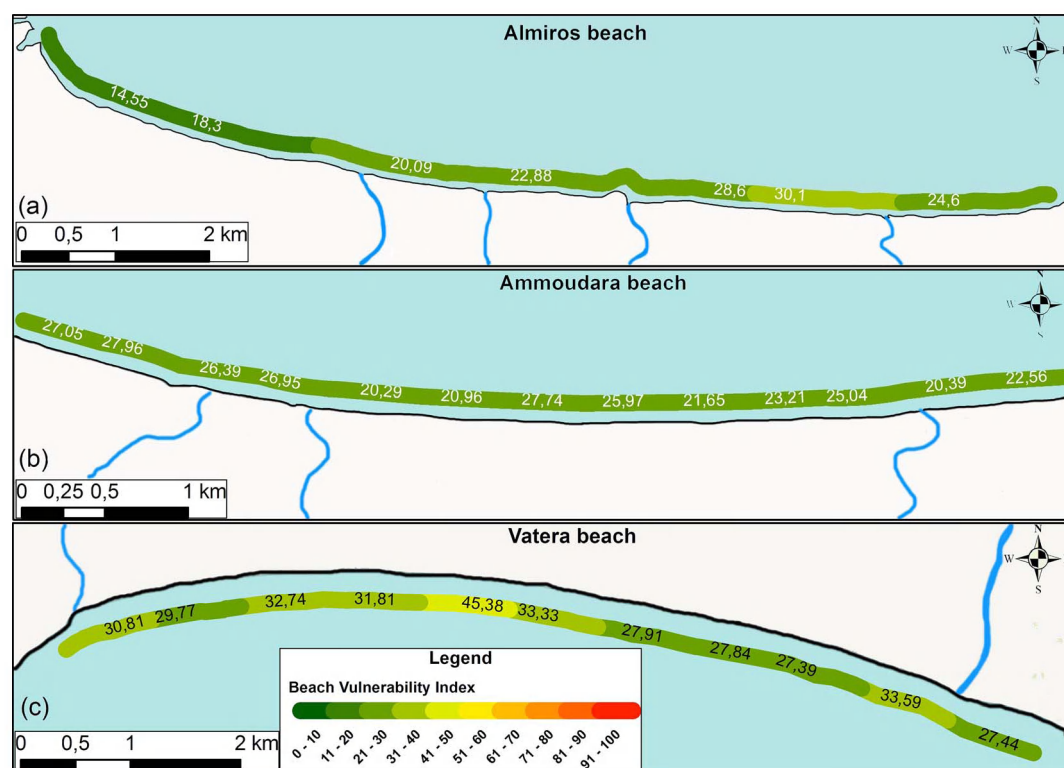


Figure 4 | Graphical presentation of the results of BVI at Almiros (a), Ammoudara beach (b) and Vatera (c). Maps were created with ArcMap 9.3.



Finally, this probabilistic evaluation of beach erosion, operating in different physio-geographical settings, we believe that it provides coastal zone managers with additional input towards risk assessment, by prioritizing beaches and/or beach sectors where coastal defences are needed, contributing, therefore, to a more effective response to erosion phenomena and to adaptation to climate change.

Methods

For the morphological analysis of the test areas topographic maps (1 : 50,000) and diagrams (1 : 5,000), hydrographic charts (1 : 5,000), aerial and satellite images imported in ArcGIS⁵⁶ were used. Detailed *in-situ* morphological measurements included 138 shore-normal beach profiles in the centre of the beach sector, extending from the maximum beach elevation to a mean water depth of 15 m. We obtained the sub-aerial and shallow parts of all profiles with a Differential Geographical Positioning System (DGPS), whilst offshore depths were obtained using a small boat equipped with an echo sounder. For the study of the textural characteristics of the coastal sediments, we collected 690 surficial sediment samples from the subaerial and subaqueous parts of the 138 profiles using a cylindrical sediment sampler 5 cm in diameter and 15 cm height. Subsequently, the upper 2–5 cm of each sample were used for the grain size analysis, according to Folk's⁵⁷ analytical procedure and using 0.5 ϕ sieve intervals. For statistical elaboration and the classification of sediment samples with respect to their grain size and texture, we have also used Folk's⁵⁷ formulae. The offshore wave climate (significant height and period) were derived from either local available data sheets (waves and wind) and/or the ERA_INTERIM⁵⁸ data base. We estimated nearshore wave characteristics (wave breaking height and angle) using a numerical model established in Matlab.

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Author contributions

G.A. developed the model and carried out the analyses as part of his PhD thesis, while S.P. had supervised the thesis that concluded in 2011. Both authors discussed the results and contributed to the preparation of the manuscript.

Additional information

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